

Stress Concentration Factors in FRP Strengthened Tubular T-joints Under Brace In-Plane Bending and Out-of-Plane Bending Moments

Alireza Sadat Hosseini^{1*}, Mohammad Reza Bahaari², Mohammad Lesani³

¹PhD Candidate, University of Tehran; a.sadat@ut.ac.ir

²Associate Professor, University of Tehran; mbahari@ut.ac.ir

³Assistant Professor, University of Tehran; m.lesani@sadra.ac.ir

ARTICLE INFO

Article History:

Received: 22 Sep. 2016

Accepted: 15 Mar. 2016

Keywords:

Tubular T-joint

FRP

SCF

IPB

OPB

Finite Element

ABSTRACT

This research is dedicated to study the relative stress concentration factors (SCF) at FRP strengthened tubular T-joint subjected to brace in-plane and out-of-plane bending moments using Finite Element Analyses performed by ABAQUS software package. Validation analysis for the finite element model of the unstiffened joint is performed against the experimental results together with the Lloyd's Register and API equations. The effectiveness of using three different types of FRP materials such as Glass/Vinyl ester, Glass/Epoxy (Scotch ply 1002) and Carbon/Epoxy (T300-5208) on enhancing the fatigue life of tubular T-joints through computing the SCFs was investigated in three schemes. At first the chord alone was strengthened in order to investigate the effects of strengthening the chord member on SCFs. In the second phase, FRP was applied only on the brace member to study the brace strengthening effects, and in the third phase, both of the chord and brace members were strengthened. Promising results derived from analysis which show that FRP strengthening method can effectively decrease the SCF values at T-joints.

1. Introduction

In offshore steel jackets circular hollow sections (CHS) are the load bearing members. CHSs are chosen due to their adequate structural performance against bending, buckling and torsion and also another advantages such as high strength-to-weight ratio, high buoyancy and lower drag coefficients [1]. T-joints are the most common and basic CHS joint configuration which are made by welding the cross section of one tube (brace) perpendicular to the undisturbed exterior surface of the other tube (chord).

Offshore steel jackets are subjected to repeated loading and unloading cycles due to the cyclic nature of wave loading. Therefore, care must be given to fatigue phenomenon in such structures. Thus, in order to ensure structural safety, prediction of fatigue life of offshore structural joints and accurate estimation of fatigue load resistance are necessary. Fatigue life of offshore structures is normally assessed using Stress-Life (S-N) curves. In this way, hot-spot stress range (HSSR) is calculated from a parameter called the stress concentration factor (SCF). According to API-Part 8.3.1 [2], "For each tubular joint configuration and each type of brace loading, SCF is defined as: The hot spot stress range (HSSR)/nominal brace stress range". Here in this research we concentrated on

estimation of SCFs at chord member. So, the HSSR is the hot spot stress range on the chord which must be divided by the nominal direct stress in the brace member to give the SCF. Having HSSR, and using S-N curves, the number of loading cycles that the structure can sustain before failure could be estimated. SCFs can be considered in unstiffened and stiffened joints. In unstiffened joints, many researchers did efforts since the 1970s and their main objective was to derive parametric equations for the SCF calculation. Lesani et al. [3] reviewed some of the strengthening techniques of CHSs. There are metallic as well as non-metallic external reinforcement schemes such as FRP strengthening technique. FRP wrapping method due to its convenience for handling and application, potentially high overall durability, corrosion resistance, light weight, superior strength-to-weight ratio, tailorability and high specific performance attributes with respect to steel, could be easier for application in areas which conventional materials may encounter durability, weight or lack of design flexibility constraints. As examples of studies on FRP strengthened steel structures, researches done by Hollaway and Cadei [4], Zhao and Zhang [5] and Zhao [6] are remarkable. FRP-strengthened CHSs with four layers of CFRP under tension loading was

investigated by Jiao and Zhao [7]. Zhao et al. [8] studied the load bearing capacity of RHSs strengthened with CFRP sheets. Lesani et al. [3] numerically investigated the failure pattern, ultimate static strength and detailed behavior of steel tubular T-joints strengthened by GFRP (Glass/epoxy) under axial brace compressive loading. Remarkable increase in joint ultimate capacity due to the combined action of steel and composite against the compressive load was observed. In addition, critical deformations and ovalization of chord member showed a descending trend up to 50% of the un-strengthened joint. Lesani et al. [9] experimentally investigated the improvement of ultimate capacity of T-joints wrapped with GFRP (Glass/vinyl ester) under static compressive loading. Increase up to 50% in the ultimate load bearing capacity of the tubular joint strengthened by FRP was observed. A numerical and experimental research program was conducted by Lesani et al. [10] on T and Y shaped CHS steel tubular connections strengthened with GFRP. This research showed numerically and experimentally that, the static strength of tubular joints could be improved significantly by the FRP wrapping technique. Based on the improvements observed in the static strength, investigation of SCF improvement and fatigue strength enhancement using the FRP wrapping technique seems to be necessary. The present research is aimed on such investigations on relative SCF values not addressed in previously studies. In this paper, the results of numerical analyses on FRP-strengthened steel tubular T-joints were used to present the effect of FRP material type on the SCF distribution along the weld toe under brace axial loading, and also the SCF values at crown and saddle positions under in-plane and out-of-plane bending moments respectively using Finite Element Analyses performed by ABAQUS software package. The finite element models of the unstiffened joints were verified against the experimental results extracted from HSE OTH 354 report [11] and the predictions of Lloyd's Register (LR) [11] and API [2] equations. The results of analysis on stiffened finite element models could address how FRP properties can affect the SCF values and accordingly the fatigue life the strengthened T-joint under three major types of loadings.

Finite Element Modeling and Verification

Here, finite element models were developed and analysis were carried out using ABAQUS software package [12]. The T-joints were chosen from JISSP project experimental results [11], JISSP joint 1.13. Three types of common FRP materials, Glass/vinyl ester, Glass/epoxy (Scotch ply 1002) and Carbon/epoxy (T300-5208), are used as strengthening material on the T-joints in order to find out how FRP wrapping affects the SCF values through the parametric study. Table 1 demonstrates the properties of the FRP materials used in the analyses. In this table, subscripts "1" and "2" stand for the fiber longitudinal and transverse directions respectively. In order to achieve accurate stresses along the chord-brace intersection, the weld profile should be modeled accurately. Here, the weld profile along the brace-chord intersection satisfies the specifications addressed in AWS [14]. In Figure 1b the weld profile section in finite element model can be seen. In this study, ABAQUS 3D brick elements are incorporated to model the joint geometry and weld profile after the sensitivity analysis. Element type C3D20 which is a 20-node quadratic brick is used to model the joint. FRP material is modeled using shell elements defined as a skin layer on the joint. ABAQUS element type S4R, which is a 4-node doubly curved thin or thick shell with reduced integration is used to model the FRP skin layers. FRP mesh elements are tied to the solid elements of the un-stiffened joint geometry, in other words, a perfect bond state was considered and no cohesive/adhesive element was modeled at the FRP and the steel substrate interface. This is not a irrational assumption since the bond between FRP layer and steel stays undisturbed in the elastic range of loading according to the experimental and numerical analysis by Lesani et al. [9].

Different sub-zone mesh generation methods was used for the weld profile, hot spot stress region, FRP wrapping area and other regions of the joint. The mesh in the hot-spot stress region is much finer than the other zones since more computational precision is required in this area. FRP wrapping areas have coarser meshes but still fine enough to ensure computational accuracy. Figure 1 shows the mesh generated for the tubular T-joint.

Table 1: FRP properties [13]

	E_1 (Mpa)	E_2 (MPa)	ν_{12}	G_{12} (MPa)	G_{13} (MPa)	G_{23} (MPa)
Glass/ Vinyl ester	28000	7000	0.29	4500	4500	2540
Glass/Epoxy (Scotch ply 1002)	38600	8270	0.26	4140	4140	3100
Carbon/Epoxy (T300-5208)	132000	10800	0.24	5700	5700	3400

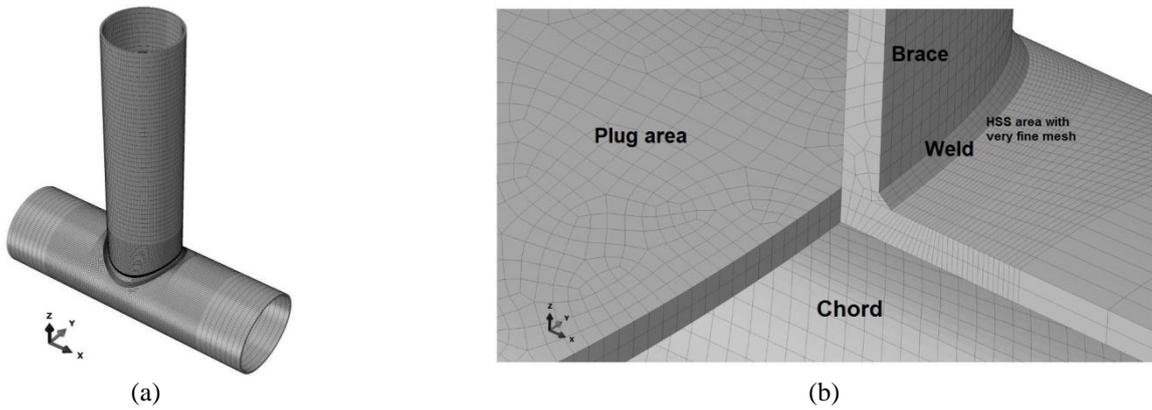


Figure 1: The mesh generated for the T-joint (JISSP joint 1.13) using the sub-zone method: a) Isometric view; b) Mesh enlargement

As previously mentioned, the tubular T-joints in this study are modeled based on the joint properties given in JISSP project [11]. Thus, all the brace and chord geometry details are modeled according to JISSP joint 1.13 [11].

The boundary conditions are chosen in such a way to represent the actual boundary conditions of the experiment. Here in the analyses, chord ends were assumed to be fixed.

In order to determine the stress concentration factors in a tubular joint, a linear elastic numerical analysis needs to be performed [15]. The Young’s modulus and Poisson’s ratio of steel are taken as 207 GPa and 0.3, respectively. In order to estimate the SCFs, the method introduced by IIW-XV-E [16] is implemented. In this method, the peak stress at the weld toe is calculated by linear extrapolating the von-Mises stresses at distances of 0.4T and 1.4T from the weld toe; where T is the thickness of the chord member. SCF is calculated by dividing the von Mises stresses at weld toe by the nominal stress in the brace. In order to validate the finite element model, Lloyd’s Register equations [11] and API [2] equations for SCF computation together with the test results published in HSE OTH 354 report for JISSP joint 1.13 were used. Table 2 summarizes the verification results at the saddle and crown points. In this table, e_1 and e_2 show the percentage of relative difference of Lloyd’s Register [11] and API [2] equations with test results respectively, and e_3 denotes the percentage of relative difference between the results of finite element model and the experiment results. According to Table 2, it is obvious that the finite element model predicts the

SCFs at crown and saddle points accurately which is in good agreement with the test results and therefore, the FE model is validated.

Where D is the chord diameter; $\alpha=2L/D$, L is the chord length; $\beta=d/D$, d is the brace diameter; $\gamma=D/2T$, T is the chord thickness; $\tau=t/T$, t is the brace thickness.

Analysis and Results

Here the analyses and results on the strengthened joints are explained. In order to investigate the stress concentration in FRP strengthened tubular T-joints under brace in-plane and out-of-plane bending moments, models were generated and analyzed using ABAQUS finite element software package. Stresses along the chord-brace intersection and correspondingly the SCF values could strongly be affected by change in FRP mechanical properties due to change in fiber and/or matrix material. The three mentioned FRP materials (Glass-vinyl ester, Glass-epoxy and Carbon-epoxy) were used for strengthening. Analyses were carried out in three phases. At first, the chord alone was strengthened in order to investigate the effects of strengthening the chord member on SCFs (Figure 2a). In the second phase, FRP was applied only on the brace member to study the brace strengthening effects (Figure 2b), and finally in the third phase, both of the chord and brace members were strengthened (Figure 2c). SCFs are calculated in Crown and Saddle points. Details and results of the analyses on SCF values in strengthened T-joints are as follow.

Table 2: Comparison of finite element results with experimental data [11] and predictions of Lloyd’s Register (LR) [11] and API [2] equations

Reference	D (mm)	α	β	γ	τ	Load / Position	Test	LR Eq.	API Eq.	FEM	e_1 (%)	e_2 (%)	e_3 (%)
JISSP joint 1.13	508	6.2	0.8	20.3	1.07	IPB / Crown	3.9	4.07	4.85	3.92	4.4	24.4	0.5
						OPB / Saddle	12.2	18.2	15.3	10.1	49.2	25.4	-17.2

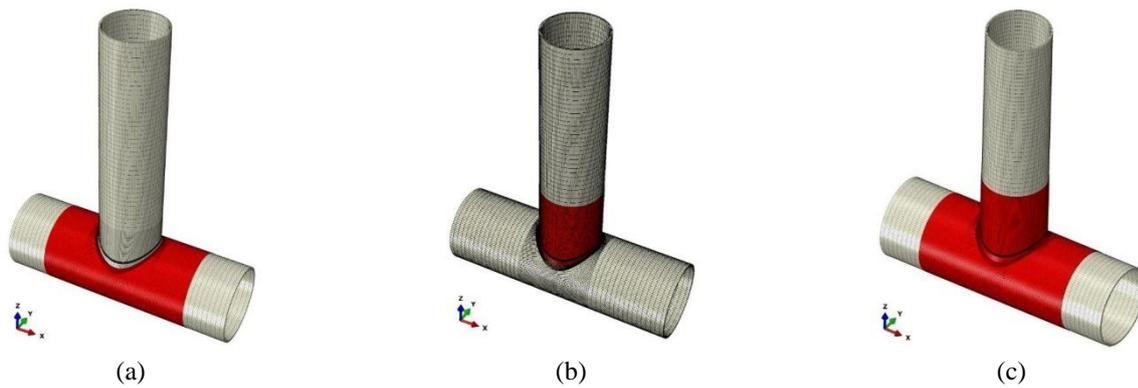


Figure 2: A typical FRP wrapped T-joint: a) Chord strengthening; b) Brace strengthening; c) Chord and brace strengthening

SCF values at crown and saddle points of the strengthened T-joint (JISSP joint 1.13) were investigated subject to IPB and OPB loadings. In all models, the applied FRP has a thickness of 1 mm, wrapping length equal to the diameter of the relative strengthened member (1D for chord member and 1d for brace member, where D and d are the chord and brace diameters respectively).

FRP materials were applied on the joint fibers in both 0° and 90° orientations. The effects of FRP material when the chord member, the brace member, and both chord and brace members were strengthened are presented in Figure 3 and 4 for IPB and OPB loadings respectively. In these figures, for instance, Ch0 stands for the strengthened chord which has 0° fiber orientation and Br90 is the strengthened brace with 90° fibers orientation. When both chord and brace members were strengthened, for instance, "Ch0Br90"

is a synthetic strengthened joint (Both chord and brace members were stiffened) with 0° fibers orientation at chord and 90° fibers orientation at brace. In other words, the chord and brace FRP fibers are along the transverse (hoop) direction. According to Figure 3, in case of IPB loading, when only the chord member was strengthened with 90° fibers, the SCF values were reduced effectively, while for OPB loading, according to Figure 4, the most decreasing effect on SCFs was observed when fibers were placed in 0° orientation in the strengthened chord. Brace strengthening had an adverse effect on SCF values. Besides, change in material properties shows more considerable alteration on SCF values when FRP fibers are in 90° and 0° in IPB and OPB loadings respectively. Focusing on these figures reveals that the strengthened brace with 0° fibers has the highest decreasing effect on the SCF amplitude.

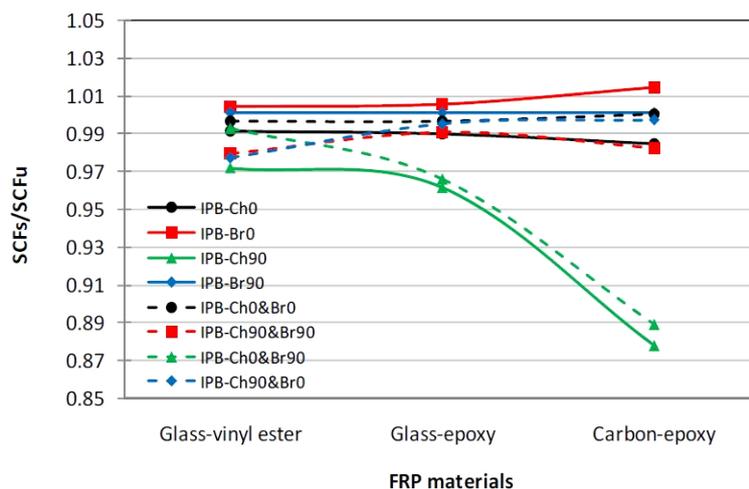


Figure 3: Ratio of SCF distribution in FRP strengthened tubular T-joint with different FRP materials to SCF distribution in un-strengthened joint under In-plane bending moment

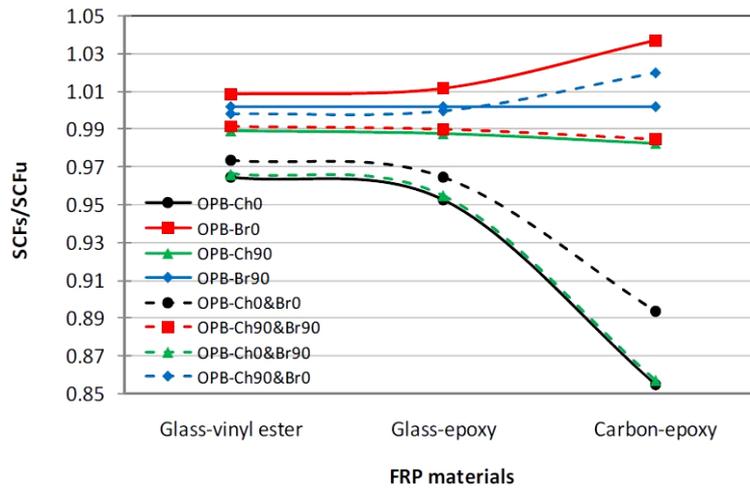


Figure 4: Ratio of SCF distribution in FRP strengthened tubular T-joint with different FRP materials to SCF distribution in un-strengthened joint under Out-of-plane bending moment

Having Figure 3 and Figure 4 along with the mechanical properties given in Table 1, one can find how the change in value of each mechanical property in two main extreme fiber orientations (0° and 90°) will affect the SCFs.

As depicted from Figure 3 and Figure 4, the effectiveness enhances by using stiffer composite material. Moreover, using carbon-epoxy material has higher effectiveness in comparison to the GFRP material on SCF reduction. Generally, it is revealed that among four different schemes of 0° and 90° fibers orientations for synthetic schemes (both chord and brace members are strengthened), when the chord member is strengthened with 0° fibers in the FRP system and the brace member with 90° (Ch0&Br90), the most reducing effect on SCF values is observed. As an example, using this scheme with Carbon-epoxy FRP material, SCFs are decreased by 11.2% under IPB loading and 14.6% under OPB loading.

Summary and Conclusions

In this research, SCF alteration at the Crown and Saddle positions of the Chord-Brace intersection area of the FRP strengthened T-joint under IPB and OPB moments was studied. Models were verified based on experimental data gathered from the JISSP project. The following conclusions were derived.

- Generally, more decrease in SCF values observed when stiffer FRP materials are used for strengthening.
- In case of IPB loading, the most decreasing effect on SCFs was observed when only the chord member is strengthened with 90° orientation of fibers.
- In case of OPB loading, use of FRP wrapping with 0° orientation of fibers only on the chord member has the most falling effect on SCFs. Brace strengthening has unfavorable results on SCFs in both IPB and OPB loading conditions, as when the

fibers are in 0° orientation, the worst effect was observed.

- Generally, results of SCFs for strengthening both chord and brace members stand between the results of individual chord or brace strengthening. As an interesting result, among the synthetic schemes (Strengthening both chord and brace members), when the chord and brace fibers are in 0° and 90° orientations respectively, the most effective result on downsizing the SCFs values was observed in both IPB and OPB loading conditions.
- Due to the better fatigue performance of CFRP materials in comparison to other composite materials such as GFRP, and based on the findings of this study, it is recommended to use CFRP composites as wrapping and strengthening material for fatigue life extension of tubular joints. Anyway, economical issues must be addressed.

Further numerical and experimental investigations would be necessary to achieve more understandings about SCF values in FRP strengthened joints.

References

- [1] Jia, J., An efficient nonlinear dynamic approach for calculating wave induced fatigue damage of offshore structures and its industrial applications for lifetime extension, "Applied Ocean Research", Vol. 30, 189-198, 2008.
- [2] API (American Petroleum Institute), Recommended practice for planning, designing and constructing fixed offshore platforms - working stress design, API RP 2A WSD, 22nd edition, 2014.
- [3] Lesani, M., Bahaari, M.R., Shokrieh, M.M., Numerical investigation of FRP-strengthened tubular T-joints under axial compressive loads, "Composite Structures", Vol. 100, 71-78, 2013.

- [4] Hollaway, L.C., Cadei, J., 2002, Progress in the technique of upgrading metallic structures with advanced polymer composites, "Progress in Structural Engineering and Materials", Vol. 4(2), 131-148.
- [5] Zhao, X.L., Zhang, L., State of the art review on FRP strengthened steel structures, "Engineering Structures", Vol. 29(8), 1808-1823, 2007.
- [6] Zhao, X.L., FRP strengthened metallic structures, "Thin Walled Structures", Special Issue, Vol. 47(10), 1019, 2009.
- [7] Jiao, H., Zhao, X.L., CFRP strengthened butt-welded very high strength (VHS) circular steel tubes, "Thin-Walled Structures", Vol. 42(7), 963-978, 2004.
- [8] Zhao, X.L., Fernando D., Al-Mahaidi R., CFRP strengthened RHS subjected to transverse end bearing force, "Engineering Structures", Vol. 28(11), 1555-1565, 2006.
- [9] Lesani, M., Bahaari, M.R., Shokrieh, M.M., Experimental investigation of FRP-strengthened tubular T-joints under axial compressive loads, "Construction and Building Materials", Vol. 53, 243-252, 2014.
- [10] Lesani, M., Bahaari, M.R., Shokrieh, M.M., FRP wrapping for the rehabilitation of Circular Hollow Section (CHS) tubular steel connections, "Thin-Walled Structures", Vol. 90, 216-234. 2015.
- [11] Health and Safety Executive (UK), OTH 354: Stress concentration factors for simple tubular joints - assessment of existing and development of new parametric formulae, Prepared by Lloyd's Register of Shipping, 1997.
- [12] ABAQUS/CAE Standard user's manual-Version 6.14-1, 2014.
- [13] Ganesh, K., Naik, N.K., "Some strength studies on FRP laminates", Journal of Composite Structures, Vol. 24, pp. 51-58, 2005.
- [14] AWS (American Welding Society), Structural welding code, AWS D 1.1:2010 (22nd Edition), Miami, FL (USA), American Welding Society, Inc, 2010.
- [15] N'Diaye, A., Hariri, S., Pluvinage, G., Azari, Z., Stress concentration factor analysis for notched welded tubular T-joints, "International Journal of Fatigue", Vol. 29, 1554-1570, 2007.
- [16] IIW-XV-E, International institute of welding sub-commission XV-E, recommended fatigue design procedure for welded hollow section joints, IIW Docs, XV-1035-99/XIII-1804-99, International Institute of Welding, France, 1999.